	IBHUVAN UNIVERSITY	Exam. Level	BE	Back Full Marks	80
			BCE	Pass Marks	32
	on Control Division	r rogramme	III/I	Time	3 hrs.
2	079 Baishakh	Year / Part		Time	<u></u>
	Subject: - Engin	eering Hydrole	ogy (CE 600	5)	
 ✓ Attempt <u>A</u> ✓ The figure. ✓ Assume su 	s are required to give their a <u>Il</u> questions. s in the margin indicate <u>Fu</u> itable data if necessary.	<u>ll Marks</u> .			
1. Discuss s	ignification of hydrology	in civil enginee	ering applica	ation with appro-	priate
examples.		2			[2:
2. a) Explai	n about IDF curve and DAI	1 41 4	onnual rain	fall at 18 surrou	nding
b) The ar	nnual rainfall at station X is are given below. Check	and the average	v of the rec	cord at station X	K and
station	ine the year in which a ch	ange in regime h	as occurred.	Determine the av	erage
determ	rainfall for the period 1952	-1970 for the cha	anged regime		
aiiiiuai					
•	Year Station. X	ual rainfall (cm) 18-stations' a	verage		
	1952 30.5	22.8	reitage		
		35			
	1953 38.9	35 30.2			
	1953 38.9 1954 43.7	35 30.2 27.4			
	1953 38.9 1954 43.7 1955 32.2	30.2			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4	30.2 27.4			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4	30.2 27.4 25.2			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4 1957 32	30.2 27.4 25.2 28.2 36.1 18.4			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4 1957 32 1958 49.3	30.2 27.4 25.2 28.2 36.1 18.4 25.1			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4 1957 32 1958 49.3 1959 28.4 1960 24.6 1961 21.8	30.2 27.4 25.2 28.2 36.1 18.4 25.1 23.6			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4 1957 32 1958 49.3 1959 28.4 1960 24.6 1961 21.8 1962 28.2	30.2 27.4 25.2 28.2 36.1 18.4 25.1 23.6 33.3			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4 1957 32 1958 49.3 1959 28.4 1960 24.6 1961 21.8 1962 28.2 1963 17.3	30.2 27.4 25.2 28.2 36.1 18.4 25.1 23.6 33.3 23.4			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4 1957 32 1958 49.3 1959 28.4 1960 24.6 1961 21.8 1962 28.2 1963 17.3 1964 22.3	30.2 27.4 25.2 28.2 36.1 18.4 25.1 23.6 33.3 23.4 36			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4 1957 32 1958 49.3 1959 28.4 1960 24.6 1961 21.8 1962 28.2 1963 17.3 1964 22.3 1965 28.4	30.2 27.4 25.2 28.2 36.1 18.4 25.1 23.6 33.3 23.4 36 31.2			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4 1957 32 1958 49.3 1959 28.4 1960 24.6 1961 21.8 1962 28.2 1963 17.3 1964 22.3 1965 28.4 1960 24.6	30.2 27.4 25.2 28.2 36.1 18.4 25.1 23.6 33.3 23.4 36 31.2 23.1			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4 1957 32 1958 49.3 1959 28.4 1960 24.6 1961 21.8 1962 28.2 1963 17.3 1964 22.3 1965 28.4 1960 24.6	30.2 27.4 25.2 28.2 36.1 18.4 25.1 23.6 33.3 23.4 36 31.2 23.1 23.4			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4 1957 32 1958 49.3 1959 28.4 1960 24.6 1961 21.8 1962 28.2 1963 17.3 1964 22.3 1965 28.4 1962 28.2 1963 17.3 1964 22.3 1965 28.4 1966 24.1 1967 26.9 1968 20.6	30.2 27.4 25.2 28.2 36.1 18.4 25.1 23.6 33.3 23.4 36 31.2 23.1 23.4			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4 1957 32 1958 49.3 1959 28.4 1960 24.6 1961 21.8 1962 28.2 1963 17.3 1964 22.3 1965 28.4 1962 28.2 1963 17.3 1964 22.3 1965 28.4 1966 24.1 1967 26.9 1968 20.6 1969 29.5	30.2 27.4 25.2 28.2 36.1 18.4 25.1 23.6 33.3 23.4 36 31.2 23.1 23.4 36 31.2 23.1 23.4 33.3			
	1953 38.9 1954 43.7 1955 32.2 1956 27.4 1957 32 1958 49.3 1959 28.4 1960 24.6 1961 21.8 1962 28.2 1963 17.3 1964 22.3 1965 28.4 1962 28.2 1963 17.3 1964 22.3 1965 28.4 1966 24.1 1967 26.9 1968 20.6	30.2 27.4 25.2 28.2 36.1 18.4 25.1 23.6 33.3 23.4 36 31.2 23.1 23.4	radiation,	relative hum	idity,

Time (h)	1	2	3	4	5	6	7
Rainfall rate (cm/hr)	1.05	1.28	0.80	0.75	0.70	0.60	0
Direct Runoff (m^3/s)	0	30	60	45	30	15	0

c) Describe the procedure of calculating evapotranspiration by Penman's method.

[5]

4. a) How rating curves are developed? Also discuss permanent and shifting controls with appropriate illustrations/figures.

b) A current meter (rating equation: V: (0.53N+0.05) m/s, where N = revolutions per second) was used to measure the vélocity at 0.6 depth. If current meter readings at various locations at a cross sections are as in the following table, calculate the discharge in the stream.

Distance from right bank (m)	0	1 .	3	5	7	9	11	12
Depth (m)	0	1.2	2.1	2.6	2.0	1.7	1.1	0.0
No of revolutions	0	39	58	112	90	45	30	0
Time (seconds)	0	100	100	150	100	100	100	0

5. a) How the base flow can be separated in hydrograph analysis? Explain with figure.

b) A catchment area of 5 km² had the following rainfall pattern.

Timeh	0	2	4	6	8	10	12	14
Cumulative rainfall cm	0	0.60	2.80	5.20	6.60	7.50	9.20	9.60

If ϕ -index is 0.40 cm/hr, construct the excess rainfall hyetograph and also find the volume of direct runoff.

- 6. a) A hydraulic structure is designed a discharge of 300 m³/s. if available flood data is for N years (reduced mean = 0.5224, reduced standard deviation 1.1124) and the mean and standard deviation for annual flood series are 140 m³/s and 50 m³/s, respectively, calculate return period for the design flood using Gumbel's method. Also estimate 90% confidence limit, if f (90%) = 1.645.
 - b) Consider rainfall storm in Melamchi during a recent flood event spanned for a period of 160 minutes. Observed records showed that during the first 30 minutes, 50 minutes, 84 minutes and 100 minutes are 25 mm, 40 mm, 70 mm and 80 mm, respectively. If catchment area of the watershed is 100 km², length of the longest flow path is 6 km, general slope of the catchment is 0.013 and runoff coefficiency is 0.6, estimate peak flow using Rational method.
- 7. What do you mean by flow routing? A drainage basin has the following characteristics: Area = 123 km², time of concentration = 14hr, storage constant = 10h and inter-isochrone area distribution as below:

Travel time (hr)	0-2	2-4	4-6	6-8	8-10	10-12	12-14	14-16	16-18
Inter-isochrone	4	10	21	24	18	20	12	9	5

Compute the flood hydrograph by using clark's method.

[2+6]

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[5]

[6]

[5+3]

[6]

[8]

[3+3]

9 TRIBHUVAN UNIVERSITY Exam. Back BE Full Marks 80 INSTITUTE OF ENGINEERING Level 32 Pass Marks Programme BCE **Examination Control Division** 3 hrs. Ш /1 Time 2078 Kartik Year/Part Subject: - Engineering Hydrology (CE 606) ✓ Candidates are required to give their answers in their own words as far as practicable. Attempt All questions. The figures in the margin indicate Full Marks. ✓ Assume suitable data if necessary. [4] 1. Explain the hydrologic cycle in nature with the help of a neat sketch. 2. Discuss three different methods of determining the average depth of rainfall over a catchment. [6] 3. Following are the rain gauge observations during a storm. Construct mass curve of precipitation, hyetograph and maximum depth-duration curve. [6] Time since commencement of storm(min) | Accumulated rainfall (cm) 0.1 5 10 0.2 0.8 15 20 1.5 25 1.8 30 2.0 2.5 35 40 2.7 2.9 45 50 3.1 4. Discuss briefly methods used to estimate evaporation from a lake. [6] 5. Following are the monthly pan evaporation data (Jan-Dec) near Kathmandu in a certain year in cm. 16.7, 14.3, 17.8, 25.0, 28.6, 21.4, 16.7, 16.7, 16.7, 21.4, 16.7, 16.7. The water spread area in a lake nearby in the beginning of January in that year was 2.80 km² and at the end of December it was measured as 2.55 km². Calculate the loss of water due to evaporation in that year. Assume a pan coefficient of 0.7. [6] 6. For ungauged basins, how would you determine the monthly flow from rainfall? Explain aný two methods. [8] 7. The high flow water surface elevations of a stream at two section 10 km apart are 306.920 m and 306.650 m. The cross-sectional area and wetted perimeters are as follows: Section Area (m²) Wetted perimeter (m) 73.3 26.80 A B 93.4 30.23 Assume n = 0.02. The eddy loss coefficient is 0.30 for gradual expansion and 0.10 for gradual contraction. Estimate the discharge at the stream. Section A is upstream of B. [8] 8. Define flood hydrograph, Direct Run of Hydrograph (DRH) and Unit Hydrograph. Write the different methods of base flow separation in hydrograph analysis. [8]

9. The ordinates of a 2-h UH are given below. Derive the ordinates of a 3-h UH by S-curve

9

Time (hr)	0	12	1	1			
Ordinates of 2-h UH	0	112	• 4	6:	. 8 :	10.	12
	0	1.12	48	78	114	72	54

10. Write the equations of flood prediction by rational and empirical methods. Also write the limitations and appropriate uses of these equations.

11. For a river the estimated flood peaks for two return periods by the use of Gumbel's [6]

Return period (yrs)	Pool A. 14 3
100	Peak flood (m ³ /s)
50	1020

What flood discharge in this river will have a return period of 500 yrs?

12. A drainage basin has the following characteristics: Area = 120 km^2 , time of concentration = 14 h, storage constant = 10 h and inter-isochrone area distribution as below:

		o ii and	inter-1soc	hrone area	dictail		o or com	centration	l
	Travel time (hr)	LOOT		chrone area	uisuidui	ion as b	elow:		
•	Inton'in T	0-2 2	4 4-6	6-8 8 10	10.10	·			
	Inter-isochrone area (km ²)	3 1	0 201	6-8 8-10	10-12	12-14	14-16	16.101	
		1	0 201	26 18.	18	. 19	-110	10-10	
	Commute the flood in 1	10/20			10	13 1	8.1	. 1	

the flood hydrograph by using Clark's method.

[8]

[8]

[6]

3 TRIBHUVAN UNIVERSITY Exam Regular INSTITUTE OF ENGINEERING BE Full Marks Level 80 **Examination Control Division** Programme BCE Pass Marks 32 2078 Bhadra Year/Part III/I Time 3 hrs. Subject: - Engineering Hydrology (CE 606) Candidates are required to give their answers in their own words as far as practicable. Attempt All questions. The figures in the margin indicate Full Marks. Graph paper will be provided. Assume suitable data if necessary. 1. What is a hydrologic cycle? Why do we need to study hydrologic cycle and water budget equation? [1+3] 2. a) What are common causes of inconsistencies in rainfall data? Please demonstrate applications of mass curves with appropriate illustrative examples. [3+3] b) Mean and standard deviations of annual rainfall estimated based on seven stations in a catchment are 143 mm and 31.6 mm, respectively. For a 7% error in the estimation of mean rainfall, do we need additional rain gauges in the catchment? If yes, what are the minimum numbers of additional rain gauges required to be established in the watershed. [5] 3. a) A reservoir of a hydropower project has a surface area of 2 km². Estimate volume of water evaporated from the reservoir in March (30 days), if temperature = 25°C, relative humidity = 70%, wind speed at 2m above the ground surface is 12 km/h, and saturation vapor pressure at 25°C is 23.76 mm of Hg. Take Meyer's coefficient as 0.36. [5] b) Daily (24-hrs) rainfall observed over a catchment of 1 km² is 10 cm. A Horton's curve with a coefficient (K) of 0.5 hr⁻¹ indicated an initial and final infiltration capacities of 0.8 cm/hr and 0.3 cm/hr, respectively. If an evaporation pan (pan coefficient = 0.7) installed in the catchment indicated 0.5 cm drop in the water level during the 24 hours of its operation, determine runoff from the catchment. [5] 4. a) During a flood flow the cross section of area of a river were measured as 60 m² for u/s and 45 m² for d/s at two sections 6 km apart. Wetted perimeters of these sections were 18 m and 14 m respectively. Elevation difference between u/s and d/s bed of the river was 0.45m. Estimate the flood discharge of the river. Take Manning's rugosity coefficient = 0.025 and eddy loss coefficient = 0.15. [8] b) Explain the methods practiced in Nepal for the estimation of runoff from an ungauged basin. [6] 5. a) Discuss how different factors affect flood hydrograph. What are various methods for baseflow separation? [3+3] b) Observed streamflow in a catchment (Area = 600 km²) from a storm event of 4 hours' duration tabulated hereunder. If baseflow is 10 m³/s, derive 4 hr unit hydrograph. [8] Time(hr) 4 0 8 12 16 20 24 28 32 36 40 44 48 Observed flow (m³/s) 10 100 230 190 130 100 70 60 50 30 20 15 10

 6. a) Using given data for annual discharge in a section of a river from year 2000 to 2009. [6+4] (i) Calculate sample mean sample standard data in
skewness.
(ii) What would be the probability of the flood of magnitude 250 m ³ /s occuring in the
Year2000200120022003200420052006200720082009Discharge (m³/s)33.931.731.559.650.538.643.428.732.051.8Reduced mean $\bar{y}_n = 0.4952$, Reduced Standard Deviation (σ_n) = 0.9497.
 b) Define the terms exceedance probability, recurrence interval and frequency factor. [6] 7. a) What are the processes that Clouble interval and frequency factor. [6]
 7. a) What are the processes that Clark's method considers in the transformation of excess precipitation to runoff? How are they achieved?
 b) Route the following hydrograph through a river reach with routing parameters K and x as 10 and 0.20, respectively. Take outflow discharge at the start of inflow flood as 5 m³/s. Also estimate attenuation and lag of peak.
Time (hr) 0 5 10 15 20 25 30 35 40 Inflow (m^3/s) 5 10 30 45 40 30 18 12 8

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1 1	TRIBHUVAN UNIVERSITY			Back	
. 1	INSTITUTE OF ENGINEERD		BE	Full Marks	80
(Ex	amination Control Div		amme BCE	Pass Marks	32
	2076 Ashwin	. Year.	Part III/I	Time	3 hrs.
-	Subject: -	Engineering I	Hydrology (CE 6	06)	
***	Candidates are required to give Attempt <u>All</u> questions. The figures in the margin indic. <u>Semi-log graph is provided.</u> Assume suitable data if necessa	ate <u>Full Marks</u> .	their own words a	as far as practicable	
	 a) In a certain catchment, in equation I=2t m³/s. If loss storage in catchment with in 	flow rate into t n the catchme			
	b) Justify the uses of Hydrolog	y in Engineerin	g Design.		[3
2.	For a station A, the recorded an	nual 24 hr maxi	num rainfalls are g	given below.	¥. [8
	Estimate the 24 hr maximum r				ided
1	semi log graph. Year 1950 51 52 53 Ppt (cm) 13.0 12.0 7.6 14.3	54 55 56	57 58 59 6		64 65
	Mean monthly temperature = Mean RH = 75% Mean Sunshine hoar = 10 hr Potential Sunshine hour = 13 Wind velocity at 2m height = Albedo = 0.028 Upper terrestrial solar radiati Latitude = 27°; Longitude = 3 Saturated vapour pressure at	.5 hr. = 8 km/hr. ion = 14.4 mm o. 86°			
	Slope of Saturated vapour pro				[8]
bj) A storm with a 15.0 cm pre distribution of storm is as fol		ced a direct runo	ff of 8.7cm. The t	ime
	Time from start (hr)	1	2 3 4	5 6 7	8
	Incremental rainfall in each hu		.35 2.25 3.45	2.7 2.4 1.5	
	Estimate the Φ -index of the s				[5]
4. a)	Define catchment. What are the	he factors affect	ng runoff from a c	catchment?	[1+5]
b)	For the purpose of discharge		in a stream by S.	lope-Area method	the [8]
	following data has been obtain	U/S Section	Middle Sectio	n D/S Section	the second se
	Area (m ²)	105.75	102.63	96.63	
	Wetted perimeter (m)	64.25	60.20	58.00	
	Gauge Reading (m)	315.5	-	315.15	_
	Manning's Roughness	0.025	0.027	0.029	
	Determine the stream dischar assuming coefficient of contra		tween U/S and L	vo sections as 260	m
				the causes of shifti	

				• •	8							
5.	a)	Define unit hydrograp	hs and	explain	the use	sofbyd	mont					
	Ь)	The ordinates of a 4	hr III	of a be		0.00	7		undas 1		[5]	
		measured at 1-hr inter 13, 9, 6, 3 and 1.5 m using the s-curve techn	/s respe	6, 36, 0 ectively	66, 91, 1 . Obtain	106, 93, 1 the on	79, 68, dinates	58, 49, of a 3 h	41, 34, 2 r UH of	27, 23, 17 the basis	n , 1	
5. 1	a)		1								[10]	
	~,	In the time series data deviations are found to $yn = 0.556$ and $S_n=1$.	or ogu	a 10 JJ	OI III /S	or 75 ye and 17	ars, the 18 m ³ /s	mean an respectiv	d standa vely. Us	urd ing		
		 Determine the peak method. Compute 90% conf confidence level res 	flood f	for 0.4%	6 probab						[4+4]	
Ъ		Explain the rational me limitations.			ining the	floods	. Also v	vrite dow	vn its			
A	dr	ainage Basin has the fo	Houing	change							[6]	
is	och	rones area distribution	oncentra	ation =	18 h, Ste	: orage ti	me cons	stant = 1:	2 h and i	inter-	[6]	
T	rav	el Time (h) -isochrone area (km ²)	0-2	2-4	4-6	6-8	8-10	1 10-12	12-14	114:01	1440	
1-		incoherry / /	3	9	20		0.10	1 10-12	112-14	14-16	16-18	

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 b) Calculate the discharge in a stream by using mid-section method from provided data. A current meter is used to measure velocity at 0.6 depth and calibrated as V=0.3N+0.004

[8]

[10]

[2]

1 0.311.0.001		52.50 1.000	5-11-1 (Al-						
Distance from right bank (m)	0	2	4	6	9	12	15	18	20
Depth (m)		0.50	1.10	.1.90	2.2	1.8	1.1	0.7	0
Number of revolutions	0	80	83	130	121	116	100	90	0
Time (s)	0	170	110	100	100	100	100	90	0

 a) Define storm hydrograph, direct runoff hydrograph and baseflow. Explain the methods to separate base flow strom hydrograph with clear sketches. [3+3]

b) Following are the ordinates of hydrograph from a catchment area of 770km² due to 6-hr rainfall. Derive the ordinates of flood hydrograph due to 3.3cm and 5.5cm effective rainfall of duration 12-hr.

t (hr)	0	6	12	18	24	30	36	42	48	54	60	66	72	
Discharge (m ³ /s)	,40	65	215	360	400	350	270	205	145	100	70	50	40	

6. a) Explain Rational method of flood predication. Also mention its limitations & uses. [3+3]
b) Analysis of the annual flood peak of river of 21 years yielded a mean of 8520m³/s and standard deviation of 3900m³/s. A proposed water control project on this river is to have an expected life of 40 years. The acceptable reliability by the design policy is 85%. [4+4]

- Using Gumbel's Method recommend the flood discharge for this project. Take y_n=0.5252 and S_n=1.0696 for 21 years.
- ii) What would the 80% confidence limit of the above flood if f(c)=1.252 at 80% confidence level.
- 7. a) What do you understand by flow routing?
 - b) A drainage basin has the following characteristics:

Area = 123 km², time of concentration=14 hr, storage constant=10h and interisochrone area distribution as below:

)-2	2-4	4-6	6-8	8-10	10-12	12-14	14-16	16-18
4	10	21	24	18	20	12	9	5
-	4	4 10	4 10 21	4 10 21 24	4 10 21 24 18	4 10 21 24 18 20	4 10 21 24 18 20 12	-2 2-4 4-6 6-8 8-10 10-12 12-14 14-16 4 10 21 24 18 20 12 9 by using Clark's IUH.

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1		AN UNIVERSITY	Exam.		Regular/	Back		
		ENGINEERING	Level	BE	Full]	Vlarks	80	
	Examination C		0	e BCE	Pass	Marks	32	
	2075 0	haitra	Year / Part	III/1	Time		3 hrs.	
			· · ·					
		Subject: - Engine	eering Hydro	logy (CE	606)	1		
	✓ Candidates are re						191	
	✓ Attempt <u>All</u> quest.	quired to give their ar	iswels in their (own words	as far as prac	ticable.		
	✓ The figures in the	margin indicate Full	Marks					
	✓ A seperate graph	paper is provided.						
	✓ Assume suitable d	ata if necessary.						
	1. Explain hydrolog Hydrology	ic cycle with neat	sketches and	instifu :+				
	Hydrology.		Directories and	Justily In	s need in E	ngineer		
	2. The rainfall denth	with time during	a atauna .t				[3+2]	
	2. The rainfall depth maximum average	intensities of the rain	a storm at a.	station is	given below.	Comp	oute	
	and plot the resultin	ng intensity duration of		ons ou min	iutes, Ihr, 2 h	ir, and		
	Time 05.00 06.20	in land			1 1 .	1	[6]	
		1.00 01.50 08.00 0	8:30 09:00 09	2:30 10:00	10:30 11:00	11:30	12:00	
	Rainfall 0 6	6 5 8	5 9 1	3 6	4 3	2	0	
1						1		
1.	3. a) Explain the wate	r budget and energy b	oudget methods	for estimation	tion of evapo	ration.	[3+3]	
	b) The mass curve (of an isolated storm in	a 500ha water	shed is as t	follows		*	
	1 ime from strat	(h) 0 2 4	6 8 10	1 10 1		8	*	
	cumulative rainfal	l (cm) 0 0.8 2.6	2.8 4.1 7.3	1108 1	10 12 1 10	1		
	If runoff measure	ed at the outlet is 0.36	51Mm ³ is base	flow action	ata the 1 ° 7	C.1	A	
	oronni und durant	in or raunan excess.	Also determin	e W-index	if the other	losses i	n	
	the Storm 15 0.1 IV.						[5+1]	
	c) Differentiate actu							
	,	al and Potential Evapo	transpirations.					
		al and Potential Evapo data of gauge and di					[3]	
	4. a) Following are the	data of gauge and di			ticular sectio	n of the	e	
	4. a) Following are the river by stream gatei) Develop a gate	data of gauge and di uging operation. ge-discharge relation	scharge collect	ted at a par	is section for		e [6+2]	
•	4. a) Following are the river by stream gaini) Develop a gain estimating the	data of gauge and di uging operation. ge-discharge relation discharge for a know	scharge collect ship for this st	ted at a par tream at th	is section for	use in	[6+2]	
	 4. a) Following are the river by stream gating i) Develop a gate estimating the correlation of the correlation of the correlation. 	data of gauge and di uging operation. ge-discharge relation discharge for a know he derived relationshi	scharge collect ship for this st	ted at a par tream at th	is section for	use in	[6+2]	
	 4. a) Following are the river by stream gather in the provided and the stimating the correlation of the to zero discharged and the strength of the st	data of gauge and di uging operation. ge-discharge relation discharge for a know he derived relationshi re.	scharge collect ship for this si wn gauge read ip? Use a =7.5	ted at a par tream at th ing. What m for the g	is section for is the coeffic gauge corresp	use in cient of onding	[6+2]	
	 4. a) Following are the river by stream gating i) Develop a gate estimating the correlation of t to zero dischargii) Estimate the discontrate the dis	data of gauge and di uging operation. ge-discharge relation discharge for a know he derived relationshi re.	scharge collect ship for this si wn gauge read ip? Use a =7.5	ted at a par tream at th ing. What m for the g	is section for is the coeffic gauge corresp	use in cient of onding	[6+2]	
· · · ·	 4. a) Following are the river by stream gation in the provided provided and the provided prov	data of gauge and di uging operation. ge-discharge relation discharge for a know he derived relationshi ge. scharge corresponding	scharge collect ship for this s wn gauge read ip? Use a =7.5 g to a gauge re	ted at a pan tream at th ing. What m for the g ading of 10	is section for is the coeffic gauge corresp).5m at this g	use in cient of onding	[6+2]	
	 4. a) Following are the river by stream gat i) Develop a gat estimating the correlation of t to zero discharg ii) Estimate the disstation. Gauge reading (m) 	data of gauge and di uging operation. ge-discharge relation discharge for a know he derived relationshi ge. scharge corresponding Discharge (m ³ /s)	scharge collect ship for this s wn gauge read ip? Use a =7.5 g to a gauge re Gauge reading	ted at a pan tream at th ing. What m for the g ading of 10	is section for is the coeffic gauge corresp 0.5m at this g harge (m ³ /s)	use in cient of onding	[6+2]	
	 4. a) Following are the river by stream gain i) Develop a gaunestimating the correlation of the to zero dischargii) Estimate the disstation. Gauge reading (m) 7.65 	data of gauge and di uging operation. ge-discharge relation discharge for a know he derived relationshi ge. scharge corresponding Discharge (m ³ /s) (15	scharge collect ship for this s wn gauge read ip? Use a =7.5 g to a gauge re Gauge reading 8.48	ted at a pan tream at th ing. What m for the g ading of 10	is section for is the coeffic gauge corresp 0.5m at this g harge (m ³ /s) 170	use in cient of onding	[6+2]	
	 4. a) Following are the river by stream gat i) Develop a gat estimating the correlation of t to zero discharg ii) Estimate the disstation. Gauge reading (m) 7.65 7.7 	data of gauge and di uging operation. ge-discharge relation discharge for a know he derived relationshi ge. scharge corresponding Discharge (m ³ /s) (<u>15</u> <u>30</u>	scharge collect ship for this s wn gauge read ip? Use a =7.5 g to a gauge re Gauge reading 8.48 8.98	ted at a pan tream at th ing. What m for the g ading of 10	is section for is the coeffic gauge corresp 0.5m at this g harge (m ³ /s) 170 400	use in cient of onding	[6+2]	
	 4. a) Following are the river by stream gat i) Develop a gat estimating the correlation of t to zero discharg ii) Estimate the disstation. Gauge reading (m) 7.65 7.7 7.77 	data of gauge and di uging operation. ge-discharge relation discharge for a know he derived relationshi ge. scharge corresponding Discharge (m ³ /s) (<u>15</u> <u>30</u> 57	scharge collect ship for this si wn gauge read ip? Use a =7.5 g to a gauge re Gauge reading 8.48 8.98 9.30	ted at a pan tream at th ing. What m for the g ading of 10	is section for is the coeffic gauge corresp 0.5m at this g harge (m ³ /s) 170 400 600	use in cient of onding	[6+2]	
	 4. a) Following are the river by stream gat i) Develop a gat estimating the correlation of t to zero discharg ii) Estimate the disstation. Gauge reading (m) 7.65 7.7 7.77 7.8 	data of gauge and di uging operation. ge-discharge relation discharge for a know he derived relationshi ge. scharge corresponding Discharge (m ³ /s) (15 30 57 39	scharge collect ship for this si wn gauge read ip? Use a =7.5 g to a gauge re Gauge reading 8.48 8.98 9.30 9.5	ted at a part tream at the ing. What m for the g ading of 10 (m) Discl	is section for is the coeffic gauge corresp 0.5m at this g harge (m ³ /s) 170 400 600 800	use in cient of onding	[6+2]	
	 4. a) Following are the river by stream gat i) Develop a gat estimating the correlation of t to zero discharg ii) Estimate the disstation. Gauge reading (m) 7.65 7.7 7.77 	data of gauge and di uging operation. ge-discharge relation discharge for a know he derived relationshi ge. scharge corresponding Discharge (m ³ /s) (<u>15</u> <u>30</u> 57	scharge collect ship for this si wn gauge read ip? Use a =7.5 g to a gauge re Gauge reading 8.48 8.98 9.30	ted at a part tream at the ing. What m for the g ading of 10 (m) Discl	is section for is the coeffic gauge corresp 0.5m at this g harge (m ³ /s) 170 400 600	use in cient of onding	[6+2]	

04 / TRIBHUVAN UNIVERSITY	Exam.	Alter Contails	Back	41. St. 20
INSTITUTE OF ENGINEERING	Level	BE	Full Marks	80
Examination Control Division	Programme	BCE	Pass Marks	32
2075 Ashwin	Year / Part	III / I	Time	3 hrs.

Subject: - Engineering Hydrology (CE606)

✓ Candidates are required to give their answers in their own words as far as practicable.

✓ Attempt <u>All</u> questions.

- ✓ The figures in the margin indicate Full Marks.
- ✓ Assume suitable data if necessary.

1.	a)	Explain double mass curve method for checking a rainfall data for consistency.	[6]
	b)	What factors should be considered in selecting a site for stream gauging station.	[4]
	c)	The catchment area of a reservoir is 1600 ha. A uniform precipitation of 8 mm/hr for 2 hour was observed on particular day. 55% run off reached the reservoir. A canal carrying a flow of $1m^3$ /s is taken from the reservoir. The rate of evaporation was 0.8 mm/h/m ² . Assuming seepage loss is 40% of evaporation loss, find the change in the reservoir level for 6 hours, if the water spread of the reservoir was 45ha.	[6]
2.	a)	Explain the different methods of determining the average rainfall over a catchment due to a storm.	[6]
	b)	Calculate the potential evapotranspiration from an area near Dharan, Sunsari in the month of april by Penmans' formula. The following data are available.	[10]
		Latitude: 26°-49'N, Mean monthly temperature : 22.5°C, Mean observed sunshine hour: 10 hr Psychrometric constant : 0.49mm of Hg/°C $e_w : 20.4mm$ of Hg, A: 1.24mm/°C b = 0.52, H _a = 14.9mm of evaporable water per day	

Mean monthly value of possible sunshine hour (N) : 12.7 hours Nature of sunshine cover: closed ground green crop, where the symbols carry their usual meanings

3. a) The mass curve of an isolated storm over a watershed is given below.

Time from start (hr)	0	0.5	1	1.5	2	2.5	3	3.5	4	4.5	5	1
Cumulative rainfall (cm)	0	0.6	1.4	1.9	2.8	3.7	5.4	6.2	7	7.8	8.2	

If the storm produced a direct run off of 3.8 cm at the outlet of the watershed, estimate the \emptyset -index of the storm and duration of rainfall excess.

b) The ordinates of a 2-h UH are given below. Derive the ordinates of a 3-h UH by Scurve method.

Time (hr)	0	2	4	6	8	10	12	14	16	18	20	22	24
Ordinates of 2-h UH	0	25	100	160	190	170	110	70	.30	20	15	6	0
. (m³/s)								-					

Calculate the flood discharge of a storm of 3h and 2h rainfall of 8 cm and 7 cm respectively. Consider \emptyset -index 0.3 cm/hr and baseflow $10m^3/s$. [10]

[6]

4. a) The ordinates of 4 hr unit hydrograph are given below.

Time (hr) 0 2 4 6 8 10 12 14 16 18 20 22 24 4-hr UH ordinates (m ³ /s) 0 9 12 28 40 52 49 36 29 20 13 10 0															
4-br IIH ordinates (m ³ /s) 0 9 12 28 40 52 49 36 29 20 13 10 0	Time (hr)	0	2								10	440	200		1
	4-hr UH ordinates (m ³ /s)	0	9	12	28	40	52	49	36	29	20	13	10	0	

The storm has successive 2 hr, 4 hr and 6 hr rainfall of 2.5, 8.0 and 9.0 cm respectively. ϕ -index is of 0.15 cm/hr and base flow of 40m³/s. Determine the 2 hr UH and resulting flood hydrograph from above storm.

- b) Annual flood peak flood of a river for 20 years yielded a mean value of 5460 m3/s and the standard deviation of 2950 m³/s, The proposed hydraulic project on this river has an expected life of 35 years and reliability of project is 87%.
 - (i) Using Gumbel's method predict the flood discharge for the project if the value of $y_n = 0.5402$ and $S_n = 1.1285$.
 - (ii) What discharge is to be adopted if the safety factor for flood magnitude is taken as 1.5 and also determine safety margin on this basis.
 - (iii)Calculate the confidence limits at 95% confidence probability f(c) = 1.96
- 5. Route the following hydrograph through a river reach for which K = 12h and X = 0.20. At the start of the inflow flood, the outflow discharge is $10m^3/s$ also find lag of peak and lag attenuation.

Time (h)	0	6	12	18	24	30	36	42	48	54	
Inflow (m ³ /s)	10	20	50	60	55	45	35	27	20	15	

[12]

[12]

[10]

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04 TRIBHUVAN UNIVERSITY	Exam.		Back -	- 16 - Je.
INSTITUTE OF ENGINEERING	Level	BE	Full Marks	80
Examination Control Division	Programme	BCE	Pass Marks	32
2074 Ashwin	Year / Part	III / I	Time	3 hrs.

Subject: - Engineering Hydrology (CE606)

- ✓ Candidates are required to give their answers in their own words as far as practicable.
- ✓ Attempt <u>All</u> questions.
- The figures in the margin indicate <u>Full Marks</u>.
- ✓ Necessary tables are attached herewith.
- ✓ Assume suitable data if necessary.
- 1. Explain water budget equation. What is the role of water budget equation in hydrology? [2+2]
- The annual rainfall at station X and the average of annual rainfall at 25 surrounding base stations in can are given below for the period of 36 years starting from 1941 [6+1+4+3]
 - i) Check whether the data of starting X is consistent
 - ii) In which year a change in regime indicated?
 - iii) Compute the mean annual rainfall for stations X at its present site for the given 36 year period first without adjustment and secondly with the data adjusted for the change in regime.
 - iv) Compute the adjusted annual rainfall at station X for the affected period.
- a) Starting from Horton's equation, derive an expression for total infiltration in time "t". Also draw graph showing infiltration and total infiltration vs time. [4+2]
 - b) Calculate the potential evapotranspiration from an area near Simara, Bara, in the month of April by Penman's formula. The following data are available. [10]

Latitude: 27'N

Elevation (from msl): 107 m

Mean monthly temperature 23°C

Mean relative humidity: 75%

Mean observed sunshine hour: 10

Wind velocity at 2 m height: 85 km/day

Nature of sunshine cover: closed ground green crop

Given:

A: 1.27mm/°C

 $H_a = 15.00 \text{ mm}$ of evaporable water per day

Mean monthly value of possible sunshine hour (N): 12.5 hours

Saturated vapour pressure at $23^{\circ}C = 21.04 \text{ mm of Hg}$

4.	a)	Calculate	the	flood	discharge	of	a	stream	by	the	slope	area	method	given	the	
		following	data	:												1

Upstream flow area = 3500 m^2

Upstream wetted perimeter = 650

Upstream velocity head coefficient = 1.17

Down stream flow area = 3250 m^2

Down stream wetted perimeter = 621 m

Down stream velocity head coefficient = 1.21

Falling difference = 0.4

Reach length = 1300 m

Manning's coefficient $\eta = 0.03$

- b) Describe about the use of current meter according to flow characteristics of channel.
- 5. a) What is Unit hydrograph? What are assumptions and limitations of UH?
 - b) In a storm, the rainfall of depth 0.7cm, 0.9cm, 0.2cm, 1.0cm occurred in four successive hours. The storm hydrograph due to this storm has following hourly ordinates:

0.5, 44.5, 110.5, 85.5, 102.8, 94.0, 38.4, 18.6, 10.9, 5.3, 2.9, 0.5 m³/s

If the average losses are 0.2cm/hr, estimate the hourly ordinates of unit hydrograph. Assume suitable value of base flow. Calculate 2-h UH using Scurre Method.

6. A river, whose annual flood peak can be represented by Gumbel distribution, has 100-years and 500-year return period flood of magnitude 9900 m3/s and 12100 m3/s

respectively. The sample size is n = 30. $y_n = 0.536$, $s_n = 1.1124$ [4+4+3+3]

- i) What is the magnitude of 200 year and 1000 year flood?
- ii) What are 95% and 80% confidence limits for 200 year and 1000 year flood if f(95%) = 1.96 and f(80%) = 1.28
- iii) A hydraulic structure of 25 year life was designed for 12300 m3/s peak flow. What is the hydrologic risk of the structure?
- iv) What peak flow should be taken into consideration if you want the structure to be 99% reliable for a structure life of 25 years.
- A drainage basin has the following Characteristics.

 $Area = 172 \text{ Km}^2$.

Storage constant = 10 hour

Time of concentration = 8 hour

The inner-isochrones area distributions are as follows

Travel Time (hr)	0-1	1-2	2-3	3-4	4-5	5-6	6-7	7-8
Inter-isochrones area (Km ²)	12	40	26	36	28	18	8	4

Determine the IUH for this catchment.

[4]

[8]

[4]

a

[12]

[2+2]

R	rtanyati ar X	Average rainfall of base stations	Year	at X	of base station
1947	163	135	1959	112	123
1942	119	III .	1960	95	142
1943	-121-	124	1961	106	92
1944	129	111	1962	81	<i>61</i>
1945	126	123	1963	116	131.
1946	120	90	1964	112	104
1947	153	138	1965	80	97
1948	172	119	1966	88	111
1949	127	108	1967	85	
1950	108	707	1968	06	92
1951	126	III.	1969	120	146
1952	190	142	0261	72	93
1953	712	112	1971	113	138
1954	E c	33	7972	82	172
1955	. 86	93	1973	116	117
1956	111	131	1974	122	152
1957	68	92	1975	73	90
1958	88	142	1976	74	104

03 TRIBHUVAN UNIVERSITY	Exam.	New Back	(2066 & Later	Batch)
INSTITUTE OF ENGINEERING	Level	BE	Full Marks	80
Examination Control Division	Programme	BCE	Pass Marks	32
2073 Shrawan	Year / Part	·III / I	Time	3 hrs.

Subject: - Engineering Hydrology (CE606)

- ✓ Candidates are required to give their answers in their own words as far as practicable.
- ✓ Attempt All questions.
- ✓ The figures in the margin indicate *Full Marks*.
- ✓ Assume suitable data if necessary.

Explain Water balance Equation and explain each process.

- 2. A storm commenced at 7:00 hours. The ordinates of the rainfall mass curve of this storm in mm as recorded by a recording rain gauge at 15 minute intervals are 0, 9.5, 17.0, 27.0, 40.5, 49.0, 63.0, 84.0, 95.0, 102.0, 110.0, 112.0 and 112.0. Plot the intensity duration graph by computing the maximum rainfall intensities for durations of 15, 30, 45, 60, 90, 120 and 180 minutes.
- 3. a) Explain energy balance equation and derive evaporation equation using Bowen's ratio. [4]
 - b) For a storm of 3 hours on 50 ha catchment, the rainfall rates are as follows:

Time of rain from beginning (min):	0	30	45	75	100	125	150	180
Rain fall rate (cm/hour):	0	2.5	3.5	2.0	4.8	5.2	1.8	5.3

If the ϕ index of this basin is 2.5 cm/hour, calculate total rainfall, runoff in (cm) and peal discharge.

- c) Explain interception and depression storage losses. How these losses are estimated during hydrological analysis. [2+2]
- 4. a) Explain how stage discharge relationship is established.
 - b) Explain the procedure of stream flow measurement by area-velocity method. Also, describe the mid section method for discharge computation using sketch and equations. [3+3]
 - b) What factors should be considered in selecting a site for stream gauging station.
- 5. a) The 3 h unit hydrograph of a basin with an area of 20 km² at one hour interval are as given below 0, 0.41, 1.38, 4, 7.72, 10.06, 9.24, 6.62, 4.57, 3.86, 2.76, 2.07, 1.38, 0.83, 0.41, 0. If rainfall excess with intensity of 2.0 cm/h for a period of 4 h followed immediately by another 3 h storm with an intensity of 1 cm/h occurs on the basin, what is the peak flow produced by this rainfall and at what time after the commencement of rainfall would this peak flow occur? Assume baseflow is negligible.
 - b) A 6 h unit hydrograph of a basin has a peak ordinate of 96 m³/s. When the base flow in the stream is 25 m³/s, and when the basin has reached its minimum infiltration capacity of 2.5 mm/h, a 6 h storm with 18.3 cm of total rainfall had occurred on the basin. What is the magnitude of the peak discharge in the flood hydrograph produced by this storm?

[4]

[4]

[4]

[12]

[6]

[4]

[10]

6. a) The annual peak discharge of a river follows the Gumbel's extreme value distribution with a mean of 10000 m³/s and a standard deviation of 3000 m³/s. What is the probability that the annual peak discharge is more than 15000 m³/s? What is the magnitude of the peak discharge with an exceedance probability of 0.1? [Hint:

$$\infty = \frac{1.28255}{\sigma}; \beta = \mu - 0.48\sigma$$

 b) Differentiate between continuous and discrete random variables. Give examples each in hydrology. Give three formulae which are used to determine the return period. [1+2+3]

7.	The ordinates	of the inflow	hydrograph at 6 h	r interval are as follows:
----	---------------	---------------	-------------------	----------------------------

Time (hrs)	0	6	12	18	24	30	36	42	48	54	60	66	72	78	84	90
Discharge (m ³ /s)	0	50	280	610	1290	1900	2130	1900	1600	1440	1060	780	500	370	220	130

The discharge over the spillway Crest and the surcharge storage above the crest for different water surface elevations are as follows:

Water surface elevation (m)	140	141	142	143	144	145	146
Outflow Discharge (m ³ /s)	0	170	482	883	1360	1905	2500
Storage $\times 10^{6} (m^3)$	0.00	15.0	35.0	60.0	95.0	140.0	240.0

Determine:

i) Maximum reservoir level

ii) Maximum outflow rate

iii) Reduction in the peak

[8]

TRIBHUVAN UNIVERSITY 04 INSTITUTE OF ENGINEERING **Examination Control Division** 2072 Chaitra

Exam.		Regular	
Level	BE	Full Marks	80
Programme	BCE	Pass Marks	32
	III / I	Time	3 hrs.

Subject: - Engineering Hydrology (CE606)

- ✓ Candidates are required to give their answers in their own words as far as practicable.
- ✓ Attempt All questions.
- ✓ The figures in the margin indicate <u>Full Marks</u>.
- ✓ Assume suitable data if necessary.
- 1. Define the following terms: hydrological cycle, runoff, water balance and catchment.
- The catchment area of a basin may be approximated as a semicircle of radius r km with 2. respect to the corordinate axis set up with its origin at the center of the circle and the x-axis coincident with the diameter the area lies in the first and second quadrants and the

position coordinates of the rain gauge stations are (0,0), $\left(\frac{r}{2},\frac{r}{2}\right)$ and $\left(\frac{-r}{2},\frac{r}{2}\right)$ km. Show

that the Thiessen weights of the gauges are given by $\frac{0.5}{\pi}$, $(0.5-0.25/\pi)$ and $(0.5-0.25/\pi)$

respectively.

- 3. a) The ordinates of a rainfall mass curve of a storm over a basin of area 850 km² measured in mm at one hour interval are 0, 10, 22, 30, 39, 45.5, 50, 55.5, 60, 64 and 68. If the infiltration during this storm can be represented by Horton's equation with $f_0 = 6.5$ mm/h, $f_C = 1.5$ mm/h and k = 0.15 /h, estimate the resulting runoff volume.
 - b) Write down Penman equation and explain all variables and constants involved in it.
- 4. a) Mention the factors that should be considered for the proper selection of stream ganging site.
 - b) Explain with sketch how you determine the stage for zero discharge.
 - c) Find the drainage density, average length of overland flow, form factor and channel [4] slope for a basin with the following data:

Area of basin (A) = 140 km^2 Distance between the outlet to the farthest point (L) = 21 km Elevation difference between the outlet and the farthest point (h) = 1090 mTotal length of channels of all order $(L_s) = 654 \text{ km}$

- 5. a) Describe the procedure of derivation of unit hydrograph from complex storms using appropriate expressions.
 - b) Given below are ordinates of a 4 h unit hydrograph of a basin in m³/s at one hour intervals.

4, 25, 44, 60, 70, 61, 52, 45, 38, 32, 27, 22, 18, 14, 11, 8, 6, 4, 2, 1 What is the area of the basin?

6. The observed annual peak flood of a river in m^3/s for a period of 20 years from 1981 to 2000 are given below:

190, 155, 298, 136, 137, 131, 140, 124, 185, 104, 91, 154, 109, 269, 164, 270, 142, 72, 130, 111.

Prepare a graph of flood peak versus the return period and hence estimate the annual peak flood with a return period of 30 years.

7. Route the following flood hydrograph through a river reach for which Muskingum coefficient k=10 h and x=0.2. At the start of inflow flood, the outflow discharge is $10 \text{ m}^3/\text{sec.}$

Providence of the probability of the property of the Providence of						and the second se				
Time (h)	0	6	12	18	24	30	36	42	48	54
		~	1 64			50				

[4]

[12]

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[4]

[4]

[6]

[8]

[6]

[14]

04	TRIBHUVAN UNIVERSITY	Exam.	New Ba	ick (2066 & Éaler	Barch
INS	TITUTE OF ENGINEERING	Level	BE	Full Marks	80
Exami	nation Control Division	Programme	BCE	Pass Marks	32
	2072 Kartik	Year/Part	Ш/І	Time	3 hrs.

Subject: - Engineering Hydrology (CE606)

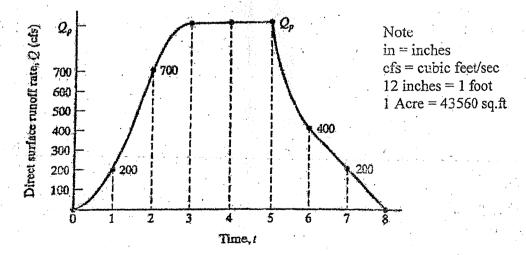
- Candidates are required to give their answers in their own words as far as practicable.
- Attempt All questions.
- The figures in the margin indicate Full Marks.
- Assume suitable data if necessary.
- 1. Explain Hydrologic cycle and water balance equations. [2+2]2. a) Explain the different methods of determining the average rainfall over a catchment due to a storm. Discuss the relative merits and demerits of the various methods. [3+3] b) Explain double mass curve test for rainfall data. 3. a) Explain briefly (i) Infiltration Capacity (ii) Φ-index lake (iii) W-index [6] b) Explain the energy budget method of estimating evaporation from a lake. [8] 4. a) Determine the stage corresponding to zero discharge from the following data of a rating curve: [8]

Stage (m)	20.80	21.42	21.95	22.37	23.00	23.52	24.00
Discharge(m ³ /s)	100	200	300	400	600	800	1000

b) Explain different methods of Stream gauge reading with sketch.

5. A hydrograph for a 4,250-acre basin is shown in the accompanying sketch. The given hydrograph actually appeared as a direct runoff hydrograph from the basin, caused by net rain falling at an intensity of 0.20 in./hr for a duration of 5 hr, beginning at t =0. [4+3+3+4]

[6]



- (a) Determine the excess release time of the basin.
- (b) What percentage of the drainage basin was contributing to direct runoff 4 hr alternation began (t = 4)?
- (c) Use your response to part (b) to determine Q_m as shown in the sketch. Do not said Q, from the drawing. γ
- (d) Note that rain continued to fall between t = 3 and t = 5. Why did the hydrograph form a plateau between i = 3 and i = 5, rather than continue to rive during these 2 hours?

- 6. a) Explain Gumble's Distribution function. Derive frequency factor (k) using Gumble's distribution.
 - b) The flood discharge for 25 and 250 years from fitted Gumbel distribution are 90 and 550 m³/sec respectively. Estimate the flood magnitudes for 50, 500 and 1000 years by Gumbel analytically.
- 7. A basin having 128 km² of drainage area has 22 hours and 14 hours of concentration time and storage constant respectively. Determine the IUH for this basin if inter-isochrones area distribution is as below:

	La Constanti de la constante d	
Area(km ²) 2 7 17 25 31 23 14	6	3

¥.

[8]

[7]

[7]

120

Regular

INSTITUTE OF ENGINEERING	Level	BE	Full Marks	80
Examination Control Division	Programme	BCE	Pass Marks	32
2071 Chaitra	Year / Part	III/I	Time	3 hrs.
Subject: - Engine	eering Hydrol	ogy (CE606)	2013년 1월 1일 - 1971년 1월 1981년 1 1월 1981년 1월 1981년 1월 1월 1981년 1월 1	
 Candidates are required to give their an Attempt <u>All</u> questions. The figures in the margin indicate <u>Full</u> Assume suitable data if necessary. 		wn words as i	far as practicable	
1. Explain.all hydrological process in Hydrological pr	lrological Cycle			[4]
2. a) Describe methods of averaging j sketches.	point rainfall (over a catchi	nent area with	neat [6]
b) Explain the energy budget method of	of estimating ev	aporation from	n a lake.	[6]
3. Calculate the potential evapotranspirat March by Penman Method.	ion for an area	over Kathma	ndu in the mon	th of [14]
The flowing data is available: Mean Monthly temp Mean RH Mean sunshine hours Potential sunshine hours Wind Velocity at 2m heig	: 10.0 : 60% : 9 h : 12.9 ht : 5 K	0		
Albedo	: 0.25			
Upper terresterial Solar rad Other values: Latitude : 28.5° Longitude : 84.5° Saturated vapor pressure at 1		= 9.2 m		
Slope of saturate vapor press	sure	= 1.24 r		
Psychrometric constant Boltzman constant		= 0.49m = 2.01*I	m/ºC E-9 mm/day	•
4. a) Compute the stream flow from fol meter is:			equation of cu	rrent [8]
V=0.035+0.74*N, where V is in m/sec and N Distance from bank (m) 0 0.	and the second se	c. 2.5 3.5	5.0 6.0 7	.0 7.5
Distance from bank (fit)00.Water Depth (m)00.		1.2 1.7		.0 7.5
No. of Revolutions 0 15	the second secon	10 120	the second s	20 0
Time (sec) 0 4	5 85	95 90	100 70 4	0 0

Exam.

04

TRIBHUVAN UNIVERSITY

b) Explain how the monthly flows from the ungauged locations are estimated from the observed rainfall data over the catchment, in Nepal.

[6]

- 5. A 2-hr unit hydrograph for a basin is shown in the sketch.
 - (a) Determine the peak discharge (in ofs) for a net rain of 5.00 in./hr and a duration of 2 hr.

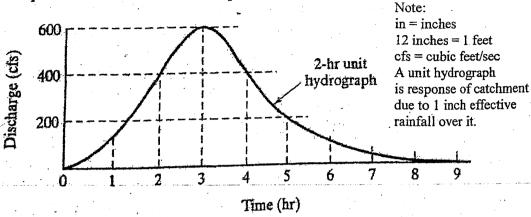
[6+3+5]

[8]

[6]

[8]

- (b) What is the total direct surface runoff (in inches) for the storm described in part (a)?
- (c) A different storm with a net rain of 0.50 in./hr lasts for 4 hr. What is the discharge at 8 p.m. if the rainfall started at 4 p.m.?



- a) If the annual flood series data for a catchment are available for N consecutive years, explain a procedure to determine a flood discharge with a return period of T, (where T>N), by using Log Pearson type III distribution method.
- b) Calculate the flood discharge using Empirical method from a catchment of area 100sqkm. The catchment has longest river of 60km. The elevation difference of the river is 20m. Rainfall runoff coefficient is 0.6 and maximum daily rainfall is 200mm.
- 7. Explain in detail time area method for estimating runoff hydrograph.

6.

03 TRIBHUVAN UNIVERSITY	Exam.	New Back (2066 & Later.	Bareli)
INSTITUTE OF ENGINEERING	Level	BE	Full Marks	80
Examination Control Division	Programme	BCE	Pass Marks	32
2071 Shawan	Year / Part	III / 1	Time	3 hrs.

5	u	biect	: - E	ngine	ering	Hvdr	ology	(CE606)

- Candidates are required to give their answers in their own words as far as practicable.
- Attempt All questions.
- The figures in the margin indicate Full Marks.
- Assume suitable data if necessary.
- Define Hydrological cycle and water balance equation. Write down a general water balance equation for a basin.
- 2. a) The shape of a catchment is in the form of a pentagon ABCDE. There are 4 rain gauge stations P,Q, R and S inside the catchment. The position co-ordinates in km are: A(0,0), B(50,75), C(100,70), D(150,0), E(75,-50), P(50,25), O(100,25), R(100,-25) and S(50,-25). If rainfalls recorded at P,O,R and S are 90, 105, 114 and 120 mm respectively, determine the mean rainfall by Thiessen Polygon method.
 - b) Explain the different types of precipitation based on lifting mechanism.
- 3. a) Calculate the free water surface evaporation in june using the Penman method from an area, whose latitude is approximately 33°N. The available data include air temperature = 30°C, wind speed at 2 m height = 10 km/h, relative humidity = 60%, mean observed sun shine hours = 12 and reflection coefficient = 0.05.
 - b) The infiltration capacity in a basin is represented by Horton's equation as $fp = 3.0 + e^{-2t}$, where fp is in cm/hr and 't' in hours. Assuming the infiltration to take place at capacity rates in a strom of 60 minutes during, estimate the depth of infiltration in (i) the first 30 minutes and (ii) the second 30 minutes of the storm.
- 4. a) Estimate the flood discharge through a 5m-wide rectangular channel for the following data. The depth of water is 2m and 1.8m at two section 500m apart. The drop in water surface elevation is 0.25 m. Manning's roughness coefficient is 0.025, Assume eddy loss to be zero.
 - b) The following data were collected for a stream at a gauging station. Compute the discharge.

Distance from one end of	Depth, d	Immersion of current meter below water surface						
water surface	(m)	at 0.6	đ	at 0.2	d	at 0.8	đ	
(m)		Rev.	Sec.	Rev.	Sec.	Rev.	Sec.	
3	1.4	12	50		1			
6	3.3			38	52	23	55	
9	5.0			40	58	30	54 '	
.12	9.0		. · .	48	60	34	58	
15	5.4			34	52	30	50	
18	3.8			35	52	30	54	
21	1.8	18	50			1		

Rating equation of current meter : v= 0.3N+0.05

5. An S- hydrograph is given such that at time t = 0, its ordinate is 1cm/h and it remains so for an indefinite period of time. Determine a 2-hour unit hydrograph. Using this unit [8+6] hydrograph, determine a 4- hour unit hydrograph.

[4]

[6] [6]

[8]

[3+3]

[8]

161

P.5

- An analysis of an annual flood., series convering the period 1890 to 1966 on a certain river shows that the 80 year flood has a magnitude of 620000 units and 1.4 year flood has a magnitude of 215000 units. Assume the annual floods are Gumbel distributed. [6+4+4]
 - i) What is the probability of having a flood as great as or greater than 440000 units?
 - ii) What is the magnitude of flood having a recurrence interval of 40 years?
 - iii) What is the probability of having 575000 units flood or a greater flood in the coming 25 years time?

[4]

[4]

- 7. a) Explain the concept of attenuation and lag of peak due to routing with sketch.
 - b) Starting from the continuity equation, obtain the equation of reservoir routing.

SATURATION, VAPOR PRESSURE OF WATER

Тепаре	rature		ation vapor ssure, e,	
°C	Ŷ	đm	mm of Hg	Slope (mm Hg/°F)
	.32	6.11	4.58	0.30
5.0	41.0	8.72	6.54	0.45
7.5	45.5	10.37	7.78	0.54
10.0	50.0	.12.28	9.21	0.60
12.5	54.5	14.49	10.87	0.71
15.0	59.0	17.05	12,79	0.80
17.5	63.5	20.00	15.00	0.95
20.0	68.0	23.38	17.54	1.05
22.5	72.5	27.25	20.44	1.24
25.0	77.0	31.67	23.76	1.40
27.5	81.5	36.71	27.54	1.61
30.0	86.0	42.42	31.82	1.85
32.5	90.5	48.89	36.68	2.07
35.0	95.0	57.07	42.81	2.35
37.5	99.51	64.46	48.36	2.62
40.0	104.0	73.14	55.32	2.95
45.0	113.0	94.91	71.20	3.66

MEAN MONTHLY SOLAR RADIATION DICIDENT AT THE EARTH'S GUTER SPACE (EXTRATERRESTRIAL RADIATION), & IN MINI OF AVERSTARE WATER/DAY, IN NORTHERN HEMISPHERE WITH L = 560 CAUC.

				1	North Int					
Month	90°	80'	70"	60*	50*	40*	30"	20°	10	0"
January	• •		_	1.4	3.6	6.0	8.5	10.8	12.8	14.5
Echnuary.			1.1	3.5	5.9	.8.3	. 10.5.	12.3	13.9	15.0
March		1.8	4.3	6.8	9.1	11.0	12.7	13:9	14.8	13.2
April	7.9	7.8	9.1	11.1	12.7	13.5	14.8	15.2	15.2	.14.7
May	14.9	14.6	13.6	14.6	15.4	15.7	16.0	15.7	15.0	13.9
June	18.1	17.8	17.0	16.5	16.7	16.7	16.5	15.8	14.8	13.4
Tuly	16.8	16.5	15:8	15.7	16.1	16.3	16.2	15.7	14.8	13.5
Acess	11.2	10.5	11.4	12.7	13.9	14.8	15.3	15.3	15.0	14.2
Schlember	2.6	£ 13'	6.3	3.5	10.5.	12.2	13.5	14.4	14.9	14.9
Detober		11	. 2.4	4.7	7.1	- 9. ³⁴	113	12.9	14.1	15.0
Hovember			0.1	1.9 ~.	.4.3	6.7	79.1	11.2	13.1	14.6
December		· · · · · · · · · · · · · · · · · · ·		0.9	3.0	5,5	7.9 .	10.3	12.4	14.3

MEAN MONTHLY VALUES	AP 4000001 - P(10)01.010	a decision of the second second
MEAN MUNIHLY VALUES	OF FUSSIBLE SURSPRINE	11631 HK 16 - 19

Latitude		iteren .	-	-					
("N)	Jan.	Feb.	Mar.	Арт.	May	June	July Aug	Sept Oct.	Nov. Dec.
0°	12.1	12:1	12.1	12.1	12.1	12.1	12.1 12.1	12.1 12.1	12.1 12.1
10"	11.6	11.8	12.1	12.4	12.6	12.7		12.9 11.9	11.7 11.5
20°	. 11.1	11.5	12.0	12.6	13.1	13.3	13.2 12.8		11.2 10.9
30°	10.4	11.1	12.0	12.9	13.7	- 14.1	13.9 13.2	12.4 11.5	
40°	9.6	10.7	11.9	13.2	14.4	15.0	14.7 13.8	12.5 11.2	
50° -	8.6	10.1	11.8	13.8	15.4-	16.4	16.0 - 14.5 -	12.7 103	10.0 9.4 9.1 8.1

03 TRIBHUVAN UNIVERSITY INSTITUTE OF ENGINEERING Examination Control Division 2070 Ashad

Exam.	New Back (2066 & Later Batch)								
Level	BE	Full Marks	80						
Programme	BCE	Pass Marks	32						
Year / Part	III / I	Time	3 hrs.						

Subject [•]	- Engineering	Hvdro	logy	(CE606)
DRUICCI.	LUIGINOVING	II) GIO.		02000/

✓ Candidates are required to give their answers in their own words as far as practicable.

- ✓ Attempt All questions.
- ✓ The figures in the margin indicate *Full Marks*.
- \checkmark Normal graph papers will be provided.
- ✓ Assume suitable data if necessary.
- Why the study of hydrology is important for engineers for planning and designing of water resources projects in Nepal? Explain the significant features of global water balance with necessary equation. [2+2]
- 2. a) In what way you can present the precipitation data? What are the benefits of each method? Explain the method of drawing Intensity Duration Frequency (IDF) curve. [3+2+3]
 - b) A catchment has seven raingauge stations. In a year the annual rainfall in cm recorded by the gauges are as follows: 130, 142.1, 118.2, 108.5, 165.2, 102.1, 146.9 for a 5% error in the estimation of the mean rainfall, calculate the minimum number of additional stations required to be established in the catchment.
- 3. A 4-hour storm occurs over a 80 km² watershed. The details of the catchment are as follows:

Sub basin	φ index	Hourly Rainfall (mm)						
(km ²)	φ index (mm/h)	1 st hour	2 nd hour	3 rd hour	4 th hour			
15	10	16	48	22	10			
25	15 .	16	42	20	8			
35	21	12	40	18	6			
5	16	15	42	18	8			

Calculate the runoff from the catchment and the hourly distribution of the effective rainfall for the whole catchment.

4. Calculate the discharge of river section as given:

Distance (m)	0	1	2	3	4	6	8	12	16	17	18	19
Depth (m)	0	1	4.3	7.2	8.5	7.4	5.6	4.7	3.5	2.1	1.4	0
Revolution / s at 0.2d	0	1.4	1.0	2.6	2.9	2.7	2.5	2.3	2.1	1.8	1.5	0
Revolution / s at 0.8d	0	.7	1.2	1.8	2.0	1.9	1.7	1.5	1.3	1.1	1.0	0

The current meter formula is v = 0.02 Ns -0.02, v = velocity (m/s) and Ns = revolution per minute.

5. In a storm the rainfall excess of 0.5 cm, 0.0 cm and 0.8 cm occurred in three successive hours. The storm hydrograph due to this storm has the hourly ordinates (Q) as given below: 0.5, 44.5, 110.5, 85.5, 102.8, 94.0, 38.4, 18.6, 10.9, 5.3, 2.9, 0.8 (cumecs). If there is a constant base flow of 0.5 cumecs, find the hourly ordinates of unit hydrograph. If 2 successive storms of 6.5 cm and 10.5 cm of 3 hours duration and φ-index of 0.2 cm/hr occurred in the same catchment, what is the peak flow from the catchment?

[14]

[4]

[14]

[9+5]

6. a) Mention the steps for the computation of flood of return period T using graphical method.

Year	1990	1991	1992	1993	1994	1995	1996	1997	1998
Peak discharge (m ³ /s)	1400	4160	2580	2910	2250	1360	2280	2540	3900
Year	1999	2000	2001	2002	2003	2004	2005	2006	
Peak discharge (m ³ /s)	3420	6170	2160	1360	5440	1340	3360	. 2800	

b) The following are the annual peak flow data (m^3/s) of a river from 1990 to 2006:

Compute flood magnitude with 50 year return period (T) using Log-Pearson type III distribution. For T = 50 year, obtain frequency factor (K_T) for the computed coefficient of skewness (C_s) using following table.

C,	0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1
KT	2.054	2.107	2.159	2.211	2.261	2.311	2.359	2.407	2.453	2.498	2.542

C,	1.2	1.4	1.6	1.8	2	2.2	2.5	3
KT	2.626	2.706	2.78	2.848	2.912	2.970	3.048	3.152

7. Explain the procedure of deriving Clark UH.

[8]

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[4]

[10]

03 TRIBHUVAN UNIVERSITY INSTITUTE OF ENGINEERING	Exam. Level	BE	Regular	20
Examination Control Divisio		BCE	Full Marks Pass Marks	80 32
2070 Chaitra	Year / Part	III / I	Time	3 hrs.
Subject: - Engi	ineering Hydrold	ogy (CE606)		
✓ Candidates are required to give their	r answers in their o	wn words as far	as practicable	
✓ Attempt <u>All</u> questions.	Ter H Marsha			
 ✓ The figures in the margin indicate <u>F</u> ✓ Assume suitable data if necessary. 	<u>ull Marks</u> .			
1. Explain different prospects of Hydro	ological study.			[4]
2. a) What can be the causes of inc		ecording the ra	infall of a sta	
Explain how it can be corrected b) The rainfall depth with time duri		ion is as given.		[4] [2+6]
		······································	11.00 11.20	
Time 6:00 6:30 7:00 7:30 8:00 Rainfall (cm) 0 7 5 8 9	0 8:30 9:00 9:3 13 10 8	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	11:00 11:30 3 1	0 12:00
i) Construct the hyetograph of this		nd 2 hours inter	 val	
ii) Compute maximum average int				this
storm and plot the resulting inten	sity duration curve	÷.		
3. a) Calculate the daily potential eva				
having the following characteris sea level, mean monthly temper				
observed sunshine hours $= 10$,				
reflection coefficient is 0.05.		5		[8]
b) Precipitation falls on a 100 km ² d	frainage basin acco	rding to the foll	owing schedul	e: [6]
Time (minute)	30 60	90 120 -		
Rainfall intensity (c		6 5		
Determine the total storm rainfall runoff is 3 cm.	I. Also, find out ϕ -	index for the ba	sin if the net s	iorm
	tion by along and			[5]
4. a) Explain the stream flow computab) Write the method of estimating n			by MIP metho	[5] od in
a Nepalese river.	-			[4]
c) What is mean by rating curve? method of drawing the rating curv				the $[1+2+2]$
	-			
5. a) The direct runoff hydrograph due such that its base is 8 hours and				
duration and intensity of the eff				
Derive and sketch a 4 hour unit h	ydrograph.			[8]
b) A 1 hour unit hydrograph is give			ours and heig	
0.25/hour. Construct an S-hydrog				[6]
6. a) Analysis of the annual flood pea and a standard deviation of 187 n				
to have an expected life of 50				
acceptable reliability of 85%.				lood
discharge for this project.				[9]
A table for reduced mean (\bar{y}_n) and	d reduced standard	deviation (S _n) i	s given below:	, ,
N 40 41	42 43	44 45		
	0.5448 0.5453	0.5458 0.5	463	
	1.1458 1.1480	1.1499 1.1	519	
b) Explain log pearson III distributio	on and its use in the	prediction of fl	ood.	[5]
-,				

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7. For what purpose time area method is used? Explain time area method using a time area histogram of a catchment and a set of effective rainfall hydrograph over it. Comments on its drawbacks. [1+5+2]

Temp	erature.	Satura . pres	Class	
°C	°F	mb	mm of Hg	Slope (mm Hg/°F)
. 0	32	6.11	4.58	0.30
5.0	41.0	8.72	6.54	0.45
7:5	45.5	10.37	7.78	0.54
10.0	50.0	12.28	9.21	0.60
12.5	54.5	14.49	10.87	0.71
15.0	59.0	17.05	12.79	0.80
17.5	63.5	20.00	15.00	0.95
20.0	68.0	23.38	17.54	1.05
22.5	72.5	27.25	20.44	1.24
25.0	77.0	31.67	23.76	1.40
27.5	81.5	36.71	27.54	1.61
30.0	86.0	42.42	31.82	1.85
32.5	90.5	48.8 9	36.68	2.07
35.0	95.0	57.07	42.81	2.35
37.5	99.5 ⁺	64.46	48.36	2.62
40.0	104.0	73.14	55.32	2.95
45.0	113.0	94.91	71.20	3.66

SATURATION VAPOR PRESSURE OF WATER

MEAN MONTHLY SOLAR RADIATION INCIDENT AT THE EARTH'S OUTER SPACE (EXTRATERRESTRIAL RADIATION). 6, IN MM OF EVALUATER WATER/DAY, IN NORTHERN HEMISPHERE WITH L = 560 CAL/G.

	North latitude ("N)									
Month	90°	80°	70°	60°	50*	40°	30°	20*	100	0°
January	• نے			1.3	3.6	6.0	8.5	10.8	12.8	14.5
Eebruary		· ·	- 1.1	3.5	5.9	.8.3	10.5	12.3	13.9	15.0
March		. 1.8	4.3	6.8	.9.1	11.0	12.7	13.9	14.8	15.2
April	7.9	7.8	9.1	11.1	12.7	13.9	14.8	15.2	15.2	.14.7
May	14.9	14.6	13.6	14.6	15.4	15:9	16.0	15.7	15.0	13.9
June	18.1	17.8	17.0	16.5	16.7	16.7	16.5	15.8	14.8	13.4
July	16.8	16.5	15.8	15.7	16.1	16.3	16.2	15.7	14.8	13.5
August	11.2	10.6	11.4	12.7	13.9	14.8	15.3	15.3	15.0	14.2
September	2.6	4.0	6.8	.8.5	10.5	12.2	13.5	14.4	14.9	14.9
October		0.2	2.4	4.7	7.1	9.3	11.3	12.9	14.1	15.0
November		. <u> </u>	0.1	1.9 -	.4.3	6.7	91	11.2	13.1	14.6
December		· · · · ·	-	0.9	3.0	5.5	7.9	10.3	12.4	14.3

MEAN MONTHLY VALUES OF POSSIBLE SUNSHINE HOURS A

atitude Rotu						Month						
"N)	Jan.	Feb.	Mar.	Apr.	May	June	July	Aug.	Sept.	- Oct.	Növ.	Dec
0°	12.1	12:1	12.1	12.1	12.1	12.1	12.1	121	12.1	12:1	12.1	12.1
10°	11.6	11.8	12.1	12.4	12.6	12.7	12.6	12,4	12.9	11.9	11.7	11.5
20°,	11.1	11.5	12.0	12.6	13.1	13.3	- 13.2 🔆	12.8	12.3	11.7	11.2	10.9
30° .	10.4	11.1	12.0	12.9	13.7	- 14.1	13.9	13.2	12.4	11.5	10.6	10.2
40°	9.6	10.7	11.9	13.2	14.4	15.0	14.7	13.8	12.5	11.2	10.0	9.4
50°	8.6	· 10.1:-	11.8	13.8	15.4	16.4	16.0-+		12.7	10.8	9.1	8.

03 TRIBHUVAN UNIVERSITY INSTITUTE OF ENGINEERING

Examination Control Division 2069 Chaitra

Exam.	Regular						
Level	BE	Full Marks	\$ 0				
Programme	BCE	Pass Marks	32				
Year / Part	III / I	Time	3 hrs.				

Subject: - Engineering Hydrology (CE606)

- ✓ Candidates are required to give their answers in their own words as far as practicable.
- ✓ Attempt <u>All</u> questions.

3.

2

- The figures in the margin indicate <u>Full Marks</u>.
- Normal graph papers will be provided.
- ✓ Assume suitable data if necessary.
- 1. What is hydrological cycle? Draw a neat sketch of the cycle showing all components. [1+3]
- 2. a) How would you determine optimum number of rain gauges to be installed in a given catchment? [6]
 - b) Explain Intensity Duration Curve and Depth Area Curve.
 - a) What is the difference between potential evapotranspiration (PET) and Actual evapotranspiration (AET)? Explain the penman's method for the estimation of PET from an area. [2+4]
 - b) The infiltration of a catchment can be represented by the equation $f = 15+50e^{-0.9t}$. If the rainfall intensity of 45mm/hr occurs continuously for 10 hour from a catchment of area $12km^2$, calculate [2+2+2+2]
 - i) Total runoff volume generated from that catchment
 - ii) Total infiltration volume at the period
 - iii) Calculate time from the start of rainfall from which runoff startediv) Show your all (above three) results in infiltration curves
- 4. a) The stage and discharge data of a river are given below. Derive the equation of rating curve (stage-discharge relationship) to predict the discharge for a given stage. Assume the value of stage for zero discharge as 161.0m.

Stage (m)	161.3	161.7	161.9	162.8	163.4	163.8	164.5	165.4	165.7
Discharge (m ³ /s)	30	120	210	450	650	825	900	1000	1050

- b) Describe the principle of slope-area method for the measurement of flood discharge in a stream. Explain the procedure to compute peak discharge using method.
- 5. A 1 hour unit hydrograph of a small catchment is triangular with peak value of 3.6 m³/s occurring at 2 hours from the start and a base time of 6 hours. Following urbanization over a period of two decades, the infiltration index φ has decreased from 0.7cm/h to 0.4cm/h. Also one hour unit hydrograph has now peak of 6.0 m³/s at 1 hours from start and time of base is 4 hours. If a design storm has intensities of 4cm/hour and 3cm/h for two consecutive one hour intervals.
 - a) Estimate the percentage increase in the peak storm runoff due to urbanization.
 - b) The volume of flood runoff due to urbanization.
- 6. The project life of headworks is 50 years. The flood discharges at risk 63.58303% is 4200 cumes. The average flood is 3500 cumec, which is derived from long term historical data using Gumbel distribution. Calculate the discharge from 500 year return period and risk 39.49939%. Prepare a Gumbel graph paper using normal arithmetic graph paper. Plot these three discharges on Gumbel paper.

7. What is linear reservoir? Explain the procedure to obtain Clark UH from time area method.

 $\frac{[2+6]}{[2+6]}$

[8]

[3+3]

[6]

[14]

[14]

03 TRIBHUVAN UNIVERSITY INSTITUTE OF ENGINEERING Examination Control Division 2068 Chaitra

Exam.	Regular					
Level	BE	Full Marks	80			
Programme	BCE	Pass Marks	32			
Year / Part	III / I	Time	3 hrs.			

Subject:	- Engineering	Hydrology	(CE 606)
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- ✓ Candidates are required to give their answers in their own words as far as practicable.
- ✓ Attempt <u>All</u> questions.

The figures in the margin indicate <u>Full Marks</u>.

Assume suitable data if necessary.

- 1. a) What is a water balance equation? Write its significance in engineering hydrology. [4]
 - b) Estimate the average depth of precipitation over the drainage basin with following data.

Isohyetals (Intervals, cm)	15-12	12-9	9-6	6-3	3-1
Inter-isohyetal area (km ²)	92	128	120	175	85

- c) A catchment has seven rain gauges. In a year the annual rainfalls in cm, recorded at these gauses are 130, 142.1, 118.2, 108.5, 165.2, 102.1 and 146.9. For a 5% error in the estimation of mean rainfall, calculate the minimum number of additional rain gauges required to be established in the watershed.
- d) Explain the use of Double Mass Curve with sketch.
- 2. a) Discuss briefly the various hydrological losses from precipitation.
 - b) Estimate daily evaporation from a lake at 30°N for April by Penman method with following mean monthly data.

T _a , Kelvin	RH, %	n, hrs	u ₂ , m/s	H _a , mm/ day	N, hrs
293	65	10	1.2	14.8	12.9

c) A storm with 20 cm of precipitation produced a surface runoff of 12cm. Estimate the Φ index of the storm.

Storm time(hr)	1	2	3	4	5	6	7	8
Rainfall (cm/hr)	0.7	2.2	3.0	4.6	3.6	3.2	2.0	0.7
~ ···	1 1	1 .	•					

- 3. a) Define rating curve and explain its uses in hydrology.
 - b) Determine the stage corresponding to zero discharge for following data of a smooth rating curve.

Stage (m)	20.80	21.42	21.95	23.37	23.00	23.52	23.90
Discharge (m ³ /s)	100	200	300	400	600	800	1000

c) Differentiate Velocity-Area and Slope-Area methods of flow estimation.

4. The ordinates of a 6-h unit hydrograph are given below. A storm had three successive 6-h intervals of rainfall magnitude of 3.0, 5.0 and 4.0cm respectively. Assuming an index of 0.20cm/h and a base flow of 30m³/s. Determine and plot the resulting hydrograph of flow.

					•									
Time (hr)	0	3	6	9	12	18	24	30	36	42	48	54	60 ·	66
6-h UH ordinate m ³ /s	Ö	150	250	450	600	800	700	600	450	320	200	100	50	0

5. a) Briefly describe the procedure of flood estimation by Log Pearson III method.

[5]

[14]

[4]

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[4]

[4]

[6]

[4]

[4]

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- b) A highway-bridge has to be designed with an expected life of 50 years and an allowable flood risk of 4%. The flood data of bridge site were well fitted to Gumbel EV distribution and discharges for 50 and 300 years return period are found to be 150 and 650m³/sec respectively. Estimate the frequency and magnitude of design flood for the bridge.
- 6. A drainage basin has area =157km², storage constant K= 9.5h and time of concentration = 7h. The following isochrones area distribution data are available. Determine the IUH of this catchment.

Time(h)	0-1	1-2	2-3	3-4	4-5	5-6	6-7
Inter-isochrone area (km ²)	10	38	20	45	32	10	2

[9]

63	TRIBHL	VAN UNIVERSITY	Exam.	Exam. Regular / I					
INST	TITUTE (OF ENGINEERING	Leyel	BE	Full Marks	80			
Exami	ination	Control Division	Programme	BCE .	Pass Marks	27			
	2067	Mangsir	Year / Part	111 / TT	Time	3 hrs.			
	e constituing per	Subject: - Eng	gineering Hyd	lrology *					

- Candidates are required to give their answers in their own wor
- Attempt any <u>Five</u> questions.
- The figures in the margin indicate <u>Full Marks</u>.
- Assume suitable data if necessary.
- 1. The following are the ordinates of the hydrograph of flow from a catchment area of 700km² due to a 6-h rainfall.

Time(Hour)	0	6	9	12	18	24	30	33	36	42	48	54	60	66	72
Discharge(m ³ /s)	40	65	140	215	360	400	350	330	270	205	145	100	70	50	40 [°]

a) Derive the ordinates of 6-h unit hydrograph

- by Calculate the flood hydrograph for two successive storms of 9.5 and 12.5 cm of 6 hours duration rainfall and an average storm loss of 0.25 cm/hr
- 2/ a) Describe the statistical approach for estimating the floods of required frequencies (design floods) when annual maximum floods of few years are available.
 - b) Compute the stream flow from following data. The calibrated equation of current meter is: V = 0.045 + 0.76N, where V is in m/sec and N is revolution/sec.

Distance from bank (m)	0	0.6	1.5	2.5	3.5	5.0	6.0	7.0	7.5
Depth (m)	0	0.3	0.75	1.2	1.7	1.3	0.7	0.3	0
No. of Revolutions	0	15	95	110	120	110	80	20	0
Time (sec)	0	45	85	.95	. 90	100	70	40.	0

- 3. a) Describe various forms of precipitation.
 - b) A-4 hour storm occurs over a 80km^2 watershed. The details of the catchment are as follows.

Sub basin(km ²)	tinday (mar (h)		Hourly Rai		
Sub Dasin(Kin)	φ index (mm/h)	1 st hour	3 rd hour	4 th hour	
15	10	-16	48	22,	10
25	15	16	42	20	8
35	21	12	40	18	6
5	16	15	42	18	8

Calculate the runoff from the catchment and the hourly distribution of the effective rainfall for the whole catchment.

4. A In a recuperation test, the static water level in an open well was depressed by pumping by 3m and it recuperated 1.5m in 1 hour. If the diameter of the well is 3.0m and the safe working depression head is 2.4m, find out the average yield of the pump.

Describe the hydro-geo-morphological characteristics of rivers with sketches.

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a) Calculate the discharge of a stream having a high water surface elevations noted at two sections A and B, 10km apart. These elevations and other salient hydraulic properties are given below.

	Section	Water Surface	Area of x-section	Hydraulic Radius
- 4	Section	elevation (m)	(m ²)	- (m).
	A	104.77	73.293	2.733
	В	104.500	93.375	3.089

The eddyless coefficient is 0.3 for gradual expansion, 0.1 for gradual contraction and Manning's roughness is 0.02.

How is the double mass curve technique used to check the consistency and adjust the rainfall at a suspicious station? Explain with sketch.

6. a) What is a rating curve? Write down a standard equation for a rating curve. Explain in detail the procedure to estimate the parameters of that rating equation. [1+1+8]

b) Prepare a Gumbel probability paper from an ordinary graph paper provided to you.

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03 TRIBHUVAN UNIVERSITY INSTITUTE OF ENGINEERING Examination Control Division 2067 Shrawan

Exam.	Back							
Level	BE	Full Marks	80					
Programme	BCE	Pass Marks	32					
Year / Part	III / II	Time	3 hrs.					

Subject: - Engineering Hydrology

 \checkmark Candidates are required to give their answers in their own words as far as practicable.

✓ Attempt any **Five** questions.

✓ The figures in the margin indicate Full Marks.

✓ Assume suitable data if necessary.

- 1. A 1-hour unit hydrograph of a small catchment is triangular with peak value of $3.6 \text{ m}^3/\text{s}$ occurring at 2 hours from the start and a base time of 6 hours. Following urbanization over a period of two decades, the infiltration index φ has decreased from 0.7cm/h to 0.4cm/h. Also one hour unit hydrograph has now peak of $6.0 \text{ m}^3/\text{s}$ at 1 hour from start and time of base is 4 hours. If a design storm has intensities of 4cm/hour and 3cm/h for two consecutive one hour intervals, estimate
 - a) the percentage increase in the peak storm runoff due to urbanization.
 - b) the volume of flood runoff due to urbanization.
- 2. a) Briefly describe the role of ground water in irrigation development.
 - b) Calculate the daily potential evaporation by Penman method using following data: Latitude = 27.5°N, Elevation = 1400m above mean sea level, mean monthly temperature = 10°C, relative humidity = 70%, mean observed sunshine hour = 7h, wind velocity at 2m height = 80 km/day, the ground surface is observed with green grass, Albedo = 0.15. Saturated vapor pressure at 10°C = 11.4mm of hg, Sloperof saturated vapor pressure = 1.24 mm/°C, Psychrometric constant = 0.49mm/°C, and Boltzman constant = 2.01×10⁻⁹ mm/day.
- a) In a 140-minute storm, the following rates of rainfall were observed in successive 20-minute intervals: 3.0, 3.0, 9.0, 6.6, 1.2, 1.2 and 6.0 mm/hr. Assume the φ index value as 3.0 mm/hr and an initial loss of 0.8mm. Determine the total rainfall, net runoff and w-index for the storm.
 - b) For a drainage basin of 600km², isohyetals drawn for a storm gave the following data:

Isohyetals intervals (cm)	15-12	12-9	9-6	6-3	3-1
Inter-isohyetals area (km ²)	92	128	120	175	85

Find the average rainfall over the basin.

- c) What is rating curve? What are the factors affecting run off? Explain.
- a) Followings are the data obtained from a stream gauging station. A current meter with a calibration equation V = (0.32N + 0.032)m/s where N = revolutions per second, was used to measure the velocity at 0.6 depth. Calculate the discharge in the stream.

Distance from right bank (m)	0	2	4	.6	9	12	15	18	20	22	23	- 24
Depth (m)	0	0.5	1.1	1.95	2.25	1.85	1.75	1.65	1.50	1.25	0.75	0
No. of Revolutions	0	80	83	131	139	121	114	109	92	85	70	. 0 -
Time (sec.)	0	180	120	120	120	120	120	120	120	120	150	0

b) Explain slope area method for estimating discharge of a stream.

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5. Using 30 years data and Gumbel's method the flood magnitudes, for return periods of 100 and 50 years for a river are found to be 1200 and 1060 m³/sec respectively. $(\bar{y}_n = 0.5362 \text{ and } S_n = 1.1124).$

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- a) Determine the mean and standard deviation of the data used.
- b) Estimate the magnitude of a flood with a return period of 500 years.
- c) What are the 95% confidence limits for this estimate if f(95%) = 1.96.
- d) What is the probability of the flood equal to or greater than a 500-year flood occuring three times in the next 10 years?
- 6. a) A 30cm well fully penetrates an unconfined aquifer of saturated depth 25m. When a discharge of 2100 lpm was being pumped for a long time, observation wells at radial distances of 30 and 90m indicated drawdowns of 5 and 4m respectively. Estimate the co-efficient of permeability and transmissibility of the aquifer. What is the drawdown at the pumping well?

b) Describe the four important hydro-geomorphological characteristics of a river.

63 TRIBHUVAN UNIVERSITY INSTITUTE OF ENGINEERING

Examination Control Division

2066 Magh

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Exam.	Regular/Back				
Level	BE	Full Marks	80		
Programme	BCE	Pass Marks	32		
Year / Part	III / H	Time	-3 hrs.		
- *					

Subject: - Engineering Hydrology

Candidates are required to give their answers in their own words as far as practicable.

- ✓ Attempt any *Five* questions.
- ✓ The figures in the margin indicate *Full Marks*.

✓ Assume suitable data if necessary.

 Unit hydrograph of 3h duration from a catchment area of 524.88 ha has peak discharge value after 9 hours from the start of the storm. It has half of the peak discharge value at 3 hours and 15 hours after the start of storm respectively. If time base (t_b) is 30 hours, constitute the unit hydrograph and calculate the flood hydrograph from the following rainfall data. Assume the Phi index (φ) as 0.333 cm/hour and base flow as 20 cumecs.

Time (hours)	0	6	9	12
Cumulative Rainfall (cm)	0	12	12	23

- 2. The project life of a headworks is 50 years. The flood discharges at the risks 63.6% and 39.5% are 4200 cumecs and 5800 cumecs respectively, which is derived from long term historical data using Gumbel distribution. Calculate the discharge from 500 years return period. Plot these three discharges on Gumbel paper.
- 3. a) Write down the factors affecting evapotranspiration.
 - b) Calculate the potential evapotranspiration for an area over Kathmandu in the month of February by Penman Method. The following data is available. [12]

Mean Monthly Temperature : 12.5°C Mean RH : 70%

Mean Sunshine Hours : 7 h

Potential Sunshine Hours : 11.9 h

Wind Velocity at 2m Height = 120 km/day

Albedo : 0.15

Upper Terrestrial Solar Radiation : 9mm of water/day

Other values:

Latitude : 28.50 Longitude : 84.50 Saturated Vapor Pressure at 12.5°C : 11.4mm of Hg Slope of Saturated Vapor Pressure : 1.24mm/°C Psychrometric Constant : 0.49mm/°C Boltzman Constant : 2.0×10⁻⁹ mm/day

- 4. a) Depth of a triangular shaped river is 3m. Its top width is 10m. The maximum depth of the river is at 4m from the one side. The top velocity of the river measured at 3.0 and 7.0m from the same side are 1.5m/s and 1.8m/s respectively. Calculate the discharge of the river.
 - b) Explain slope area method for discharge calculation.
 - (a) Explain different types of rain gauges with neat sketch.
 - b) There are four rain gauges at four corner of a rectangle. The two sides of rectangle are 100km and 150km. The yearly rainfalls of four gauges are 1200mm, 1300mm, 1500mm and 1100mm respectively. Calculate the average rainfall by Theissen polygon method.

c) Explain Φ (Phi)-index and W-index.

a) Explain hydrological cycle.

- b) Define floods. Enumerate the causes, effects and mitigation of floods.
- c) In a recuperation test, the static water level in an open well was depressed using pumps by 3m and it recuperated 1.5m in 1 hour. If the diameter of the well is 3.0m and the safe working depression head is 2.4m, find out the average specific yield of the soil and specific capacity of the soil.

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